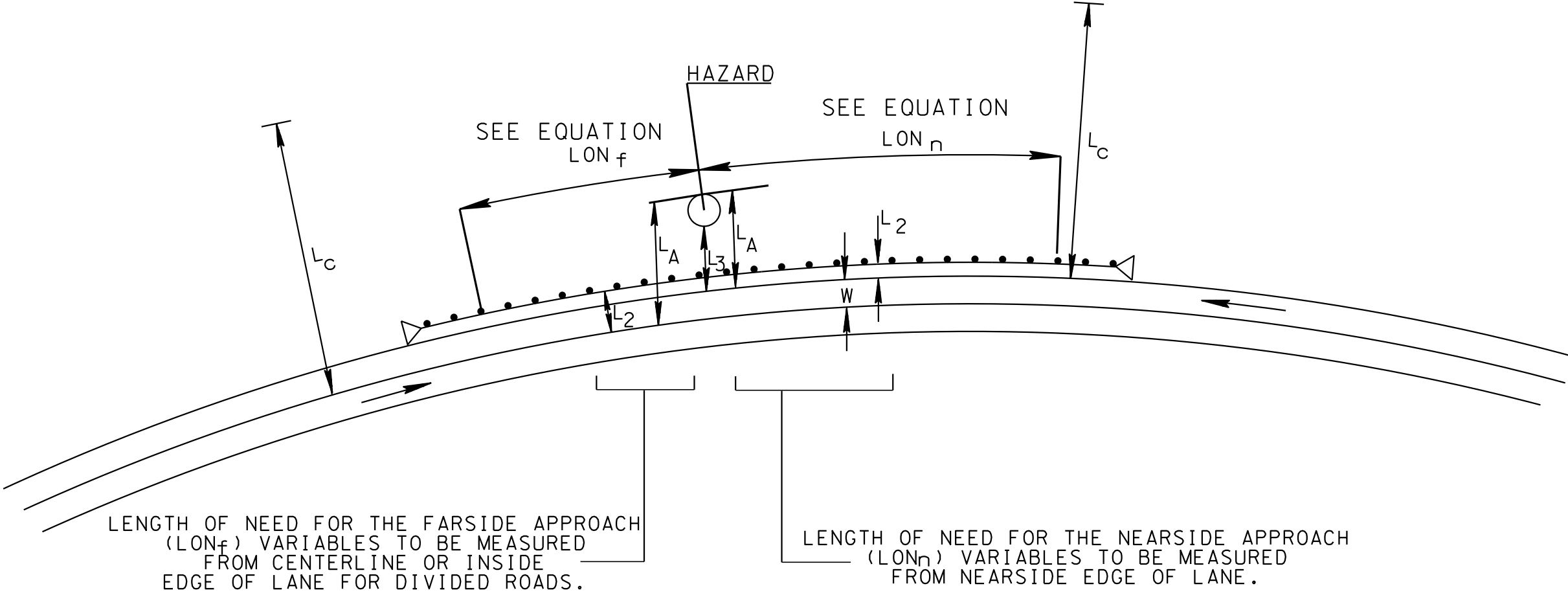


FIGURE A
LENGTH OF NEED DISTANCE
FOR CURVED ROADS

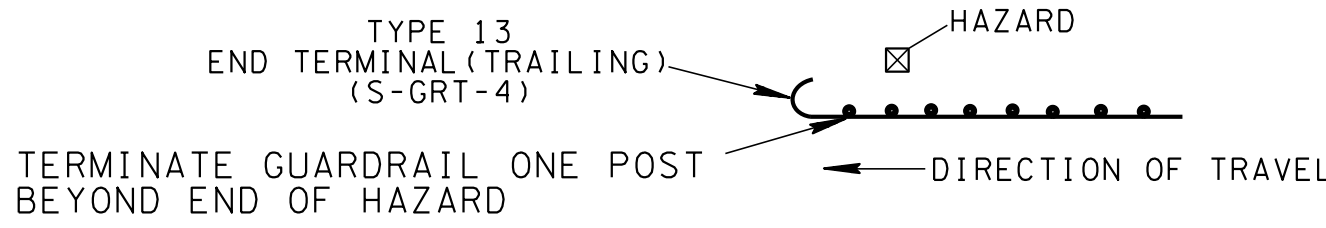


LENGTH OF NEED FOR CURVED ROADS CALCULATION

$LON = \pi AK / 180$	LEGEND L_C = THE CLEAR ZONE DISTANCE AS DETERMINED BY THE CURVE EQUATION ON S-CZ-1 L_A = DISTANCE FROM EDGE OF TRAVELED WAY TO THE LATERAL EXTENT OF HAZARD. NOTE THAT L_A SHOULD NEVER EXCEED THE "CLEAR DISTANCE" (L_C). L_2 = DISTANCE FROM EDGE OF TRAVELED WAY TO BARRIER. L_3 = DISTANCE FROM EDGE OF PAVEMENT TO NEAR FACE OF HAZARD. R = HORIZONTAL CURVE RADIUS W = WIDTH OF LANES (DISTANCE BETWEEN CENTERLINE AND EDGE OF TRAVELED WAY) FOR THE FAR SIDE LON CALCULATION $W = 0$
CALCULATION STEPS 1. CALCULATE A, B, & H USING KNOW INFORMATION 2. CALCULATE FOR I & J USING A, B & H 3. SOLVE FOR K 4. SOLVE FOR LON	INTERMEDIATE CALCULATIONS $A = R + W + L_2$ $B = R + W$ $H = R + W + L_A$ $I = \arcsin(B/H)$ $J = \arcsin(B/A)$ $K = J - I$

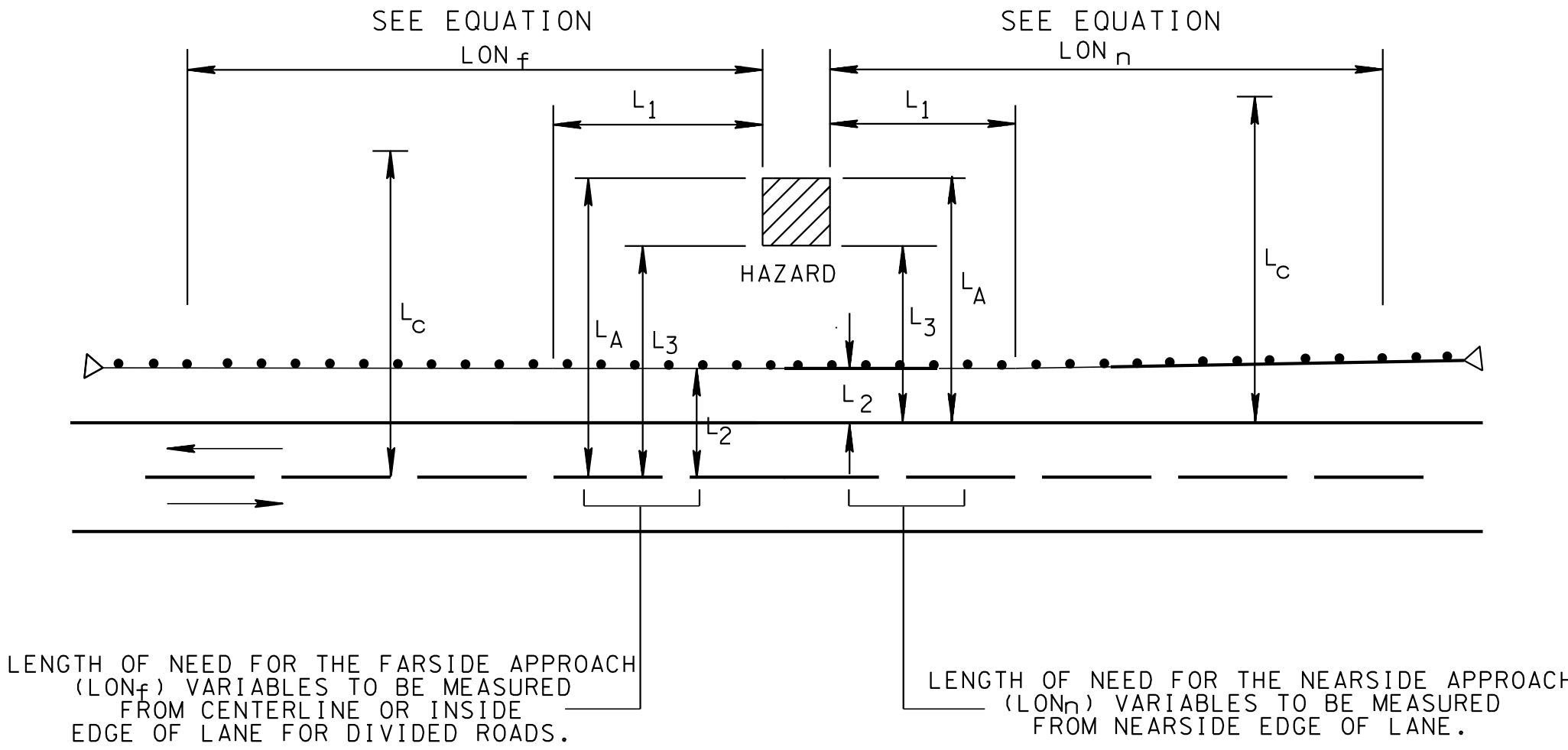
*NOTE: THE EQUATION FOR LON FOR THE NEAR SIDE AND FAR SIDE APPROACHES IS THE SAME. THE ONLY DIFFERENCES ARE $W=0$ FOR THE FAR SIDE AND HOW THE VARIABLE ARE MEASURED AS NOTED ON FIGURE A

FIGURE C
TRAILING END GUARDRAIL TERMINALS



NOTE: MAY ONLY BE USED FOR DIVIDED ROADWAYS, ONE WAY ROADS, OR TWO WAY MULTI-LANE ROADS WHERE THE LANES ON THE NEAR SIDE ARE WIDER THAN THE CLEAR ZONE FOR THE OPPOSING DIRECTION TRAFFIC.

FIGURE B
THE LENGTH OF NEED DISTANCE
FOR TANGENT ROADS



LENGTH OF NEED FOR TANGENT ROADS CALCULATION

$* \begin{matrix} L_{ON\ f} \\ \text{OR} \\ L_{ON\ n} \end{matrix} = \frac{L_A - L_2 - 0.75}{L_A / L_R}$ NOTE: Ⓐ 0.75 ACCOUNTS FOR FLARE RATE OF TERMINAL Ⓑ SEE ROADSIDE DESIGN GUIDE SECTION 5.6.4 FOR ADDITIONAL INFORMATION	LEGEND L_C = THE CLEAR ZONE DISTANCE AS DETERMINED ON S-CZ-1 L_A = DISTANCE FROM EDGE OF TRAVELED WAY (EDGE OF PAVEMENT) TO THE LATERAL EXTENT OF HAZARD. NOTE THAT L_A SHOULD NEVER EXCEED THE "CLEAR DISTANCE" (L_C). L_2 = DISTANCE FROM EDGE OF TRAVELED WAY TO BARRIER. L_R = RUNOUT LENGTH (SEE TABLE BELOW FOR VALUE). L_3 = DISTANCE FROM EDGE OF PAVEMENT TO NEAR FACE OF HAZARD.
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*NOTE: THE EQUATION FOR LON FOR THE NEAR SIDE AND FAR SIDE APPROACHES IS THE SAME. THE ONLY DIFFERENCE IS HOW THE VARIABLE ARE MEASURED AS NOTED ON FIGURE B

RUNOUT LENGTHS (L_R) FOR
BARRIER DESIGN (FT)

DESIGN SPEED (MPH)	DESIGN TRAFFIC VOLUME (ADT)			
	OVER 10000 VPD	5000-10000 VPD	1000-5000 VPD	UNDER 1000 VPD
70	360	330	290	250
60	300	250	210	200
50	230	190	160	150
40	160	130	110	100
30	110	90	80	70

GENERAL NOTES

- Ⓐ EVERY LOCATION WHERE GUARDRAIL IS REQUIRED MUST BE INVESTIGATED SEPARATELY. THE HAZARD MUST BE IDENTIFIED AND THE "POINT OF NEED" CALCULATED TO DETERMINE THE BEST TREATMENT FOR PROTECTION OF VEHICLES FROM THE HAZARD.
- Ⓑ THE THIRD POST FROM THE END TREATMENT SHALL BE PLACED AT THE END OF THE LENGTH OF NEED EXCEPT FOR TRAILING ENDS AS SHOWN IN FIGURE C.
- Ⓒ WHEN DESIRABLE CLEAR ZONE LINE FALLS INSIDE THE FILL SLOPE LINE (FOR 3:1 OR STEEPER), EXTEND THE CLEAR ZONE TO THE TOE OF THE SLOPE.
- Ⓓ THIS DRAWING IS FOR TYPICAL ROADSIDE OBSTACLES OR STEEP SLOPES IN THE CLEAR ZONE. SEE THE FOLLOWING SAFETY PLAN DRAWINGS FOR THESE SPECIAL CASES:
S-PL-2: HAZARDS NEAR INTERSECTIONS
S-PL-3: TYPICAL BRIDGE ENDS
S-PL-4: BRIDGE PIERS IN CLEAR ZONE
S-PL-5: BRIDGE ENDS IN MEDIANS
S-PL-6: NARROW MEDIAN PROTECTION
- Ⓔ IF $L_3 - L_2$ IS LESS THAN 4.0 FEET USE CONCRETE MEDIAN BARRIER INSTEAD OF GUARDRAIL.
- Ⓕ THE MINIMUM INSTALLATION DISTANCE IS EQUAL TO THE $L_{ON\ f} + L_{ON\ n} +$ THE LENGTH OF THE HAZARD